



<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 1
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

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General Wind Load

1388-3 Glass Adaptor and Spigot

Analysis By	Checked By
R.F.	T.S.

0	08/05/2020	T.S.	Issued
<b>Revision</b>	<b>Date</b>	<b>Issued By</b>	<b>Comment</b>



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### Introduction/Actions/Result Summary:

#### Introduction:

TSA was instructed by Concorde to provide a matrix of wind load for the glass adaptor and spigot.

#### Actions – Loading Considerated:

Infill load = 0.42kN	(per calculation)
Infill load = 1.0kN	(Table NA.5 IS1991-1-1:2002)
Infill load = 1.5kN	(Table NA.5 IS1991-1-1:2002)
Infill load = 2.0kN	(Table NA.5 IS1991-1-1:2002)

#### Assumption:

Concrete Grade = C30/37

#### Result Summary:

Glass Analysis					
Case Study	Glass (mm)	Interlayer	Wind Load - Qw (kN/m)	Height glass (m)	Glass Deflection (mm)
1	15		1.00	1.4	13.42
2	17.52	PVB	1.00	1.4	22.68
3	21.52	PVB	1.50	1.4	19.69
4	15		0.42	1.192	8.422
5	15		1.50	1.11	15.46

**NOTE:** All deflection < 25mm and therefore acceptable.

Connection To Concrete		
Case Study	Fischer	Edge (mm)
5	M10 FAZ II 10/10	100

Connection To Mild Steel		
Case Study	Fischer	Edge (mm)
5	M10x30mm Grade 8.8	70

Connection To Wood		
Case Study	Fischer	Edge (mm)
5	Rampa SKL M10x60mm	-



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## Glass Strength

### Wind Loading:

10min duration, Multiple Gust Storm =>  $k_{mod} = 0.74$

$$f_{gd} = (k_{mod})(k_{sp})(f_{gk})/\gamma_{ma} + k_v(f_{bk}-f_{gk})/\gamma_{mv}$$

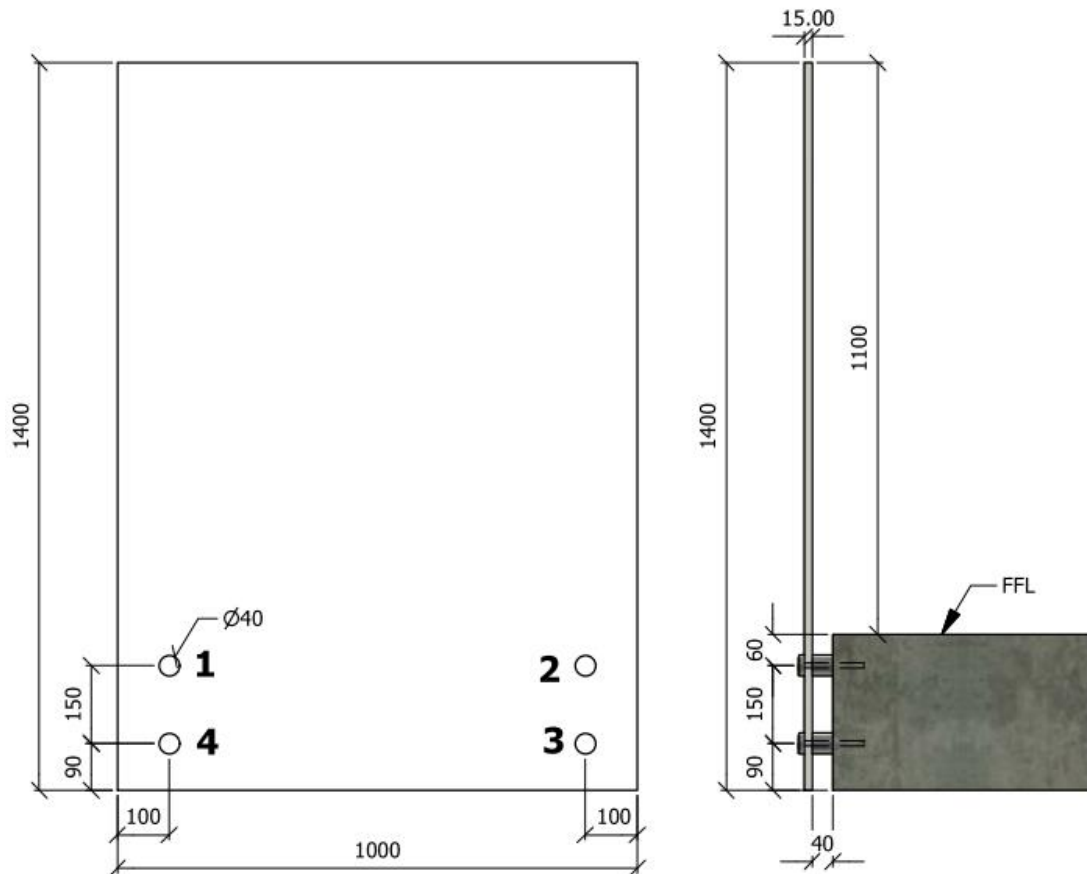
$$f_{gd} = (0.74)(1.0)(45)/1.6 + 1.0(120-45)/1.2$$

$$f_{gd} = \underline{83.3N/mm^2}$$



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Case Study 01: 15mm Tough – 1.0x1.40m – 1.0kN/m<sup>2</sup>:

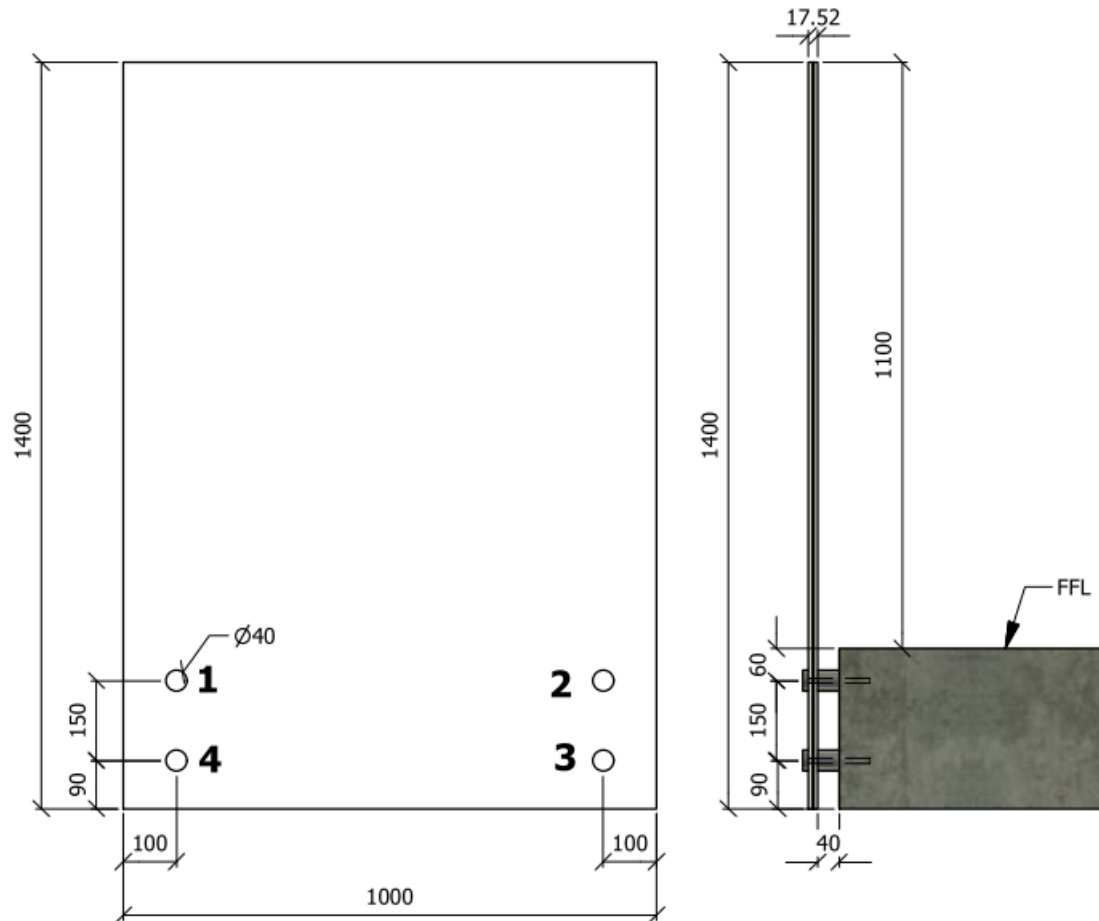


**NOTE:**

- Deflection on the glass 13.42mm = **OK in deflectionnessa**

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Case Study 02: 17.52mm (TLT) – 1.0x1.40m – 1.0kN/m<sup>2</sup>:



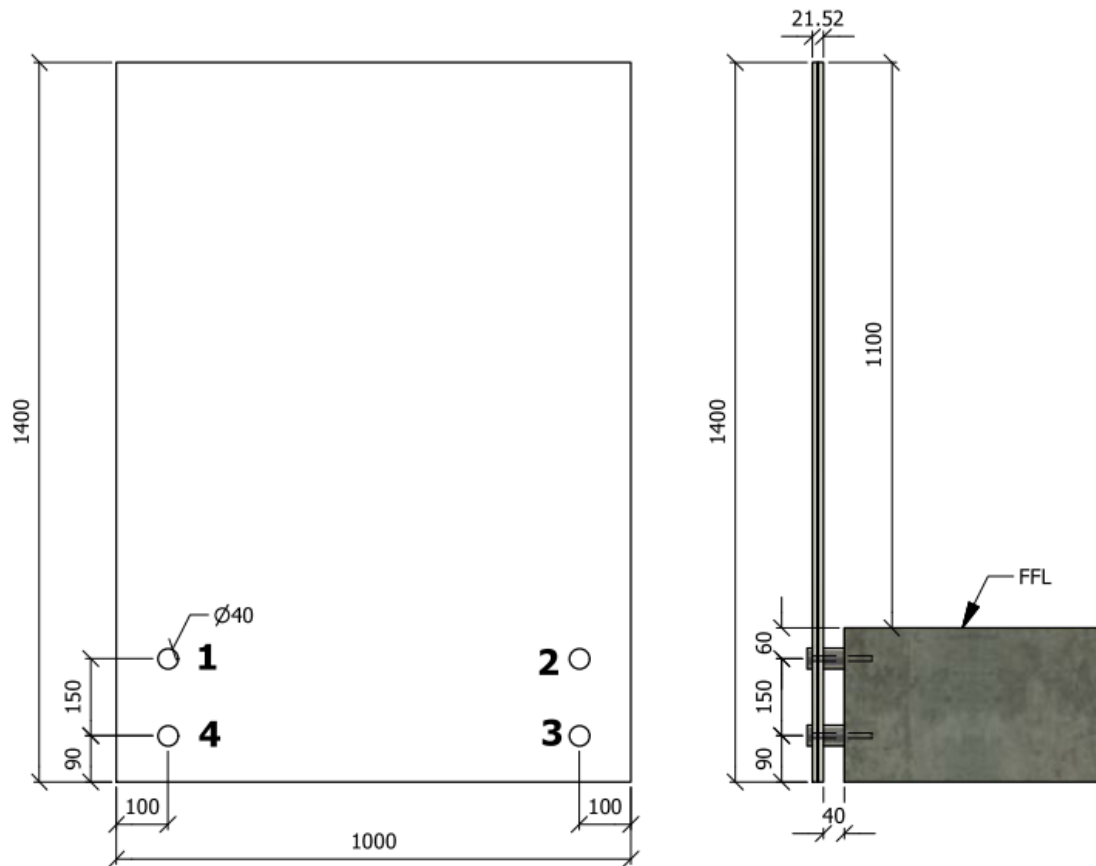
**NOTE:**

- Deflection on the glass 22.68mm = **OK in deflection**



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Case Study 03: 21.52mm (TLT) – 1.0x1.40m – 1.5kN/m<sup>2</sup>:

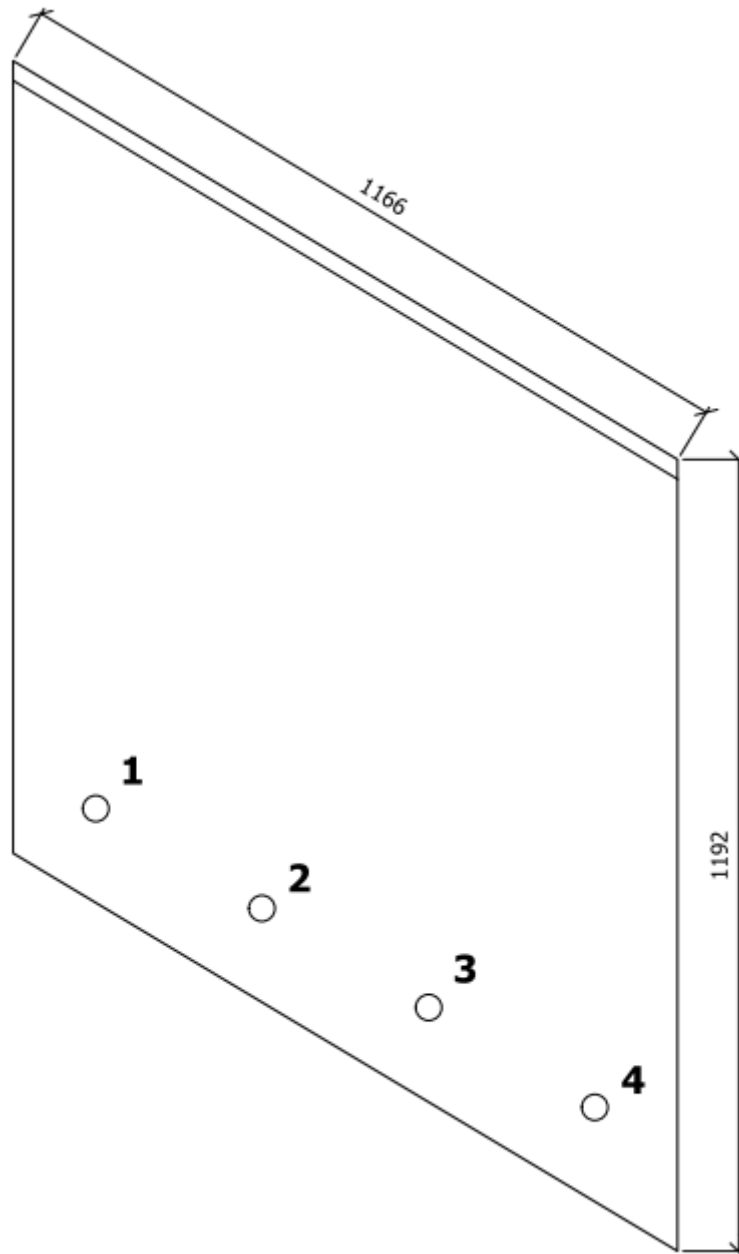


**NOTE:**

- Deflection on the glass 19.69mm = **OK in deflection**

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Case Study 04: 15mm Tough – 1.166x1.192m – 0.42kN:

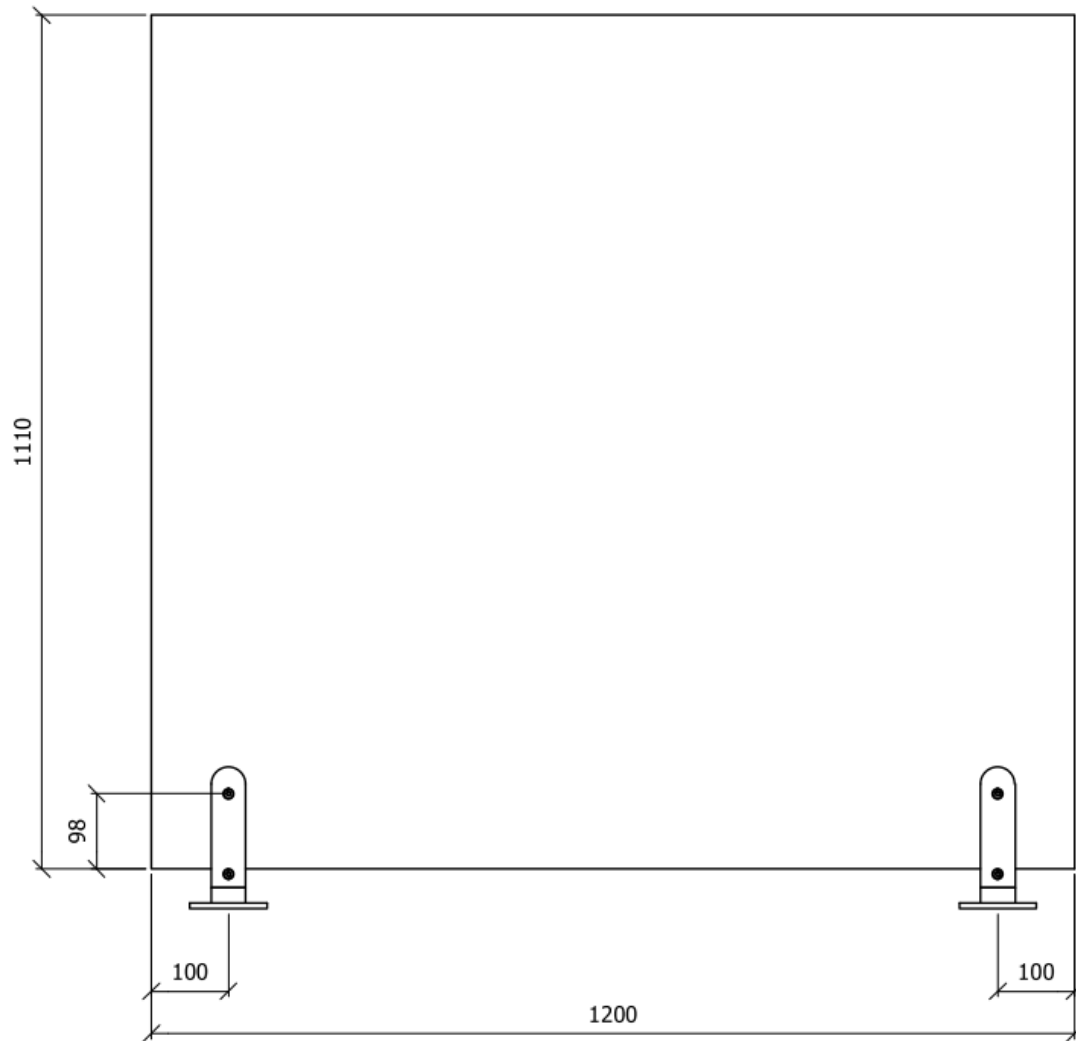


**NOTE:**

- Deflection on the glass 8.422mm = **OK in deflection**

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Case Study 05: 15mm Tough – 1.20x1.11m – 1.5kN/m<sup>2</sup>:



**NOTE:**

- Deflection on the glass 15.46mm = **OK in deflection**

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### Glass Analysis:

Case Study 01: 15mm Tough – 1.0x1.40m – 1.0kN/m<sup>2</sup>:

### Glass Analysis - Bending Stress of Glass Panel due to 1.0kN/m<sup>2</sup> Infill Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.0kN/m<sup>2</sup> Infill Loading
- 15mm Tough Glass analysed, horizontally toughened Laminated
- Bending Stress analysed based on glass panel of 1.0m x 1.40m

### Result:

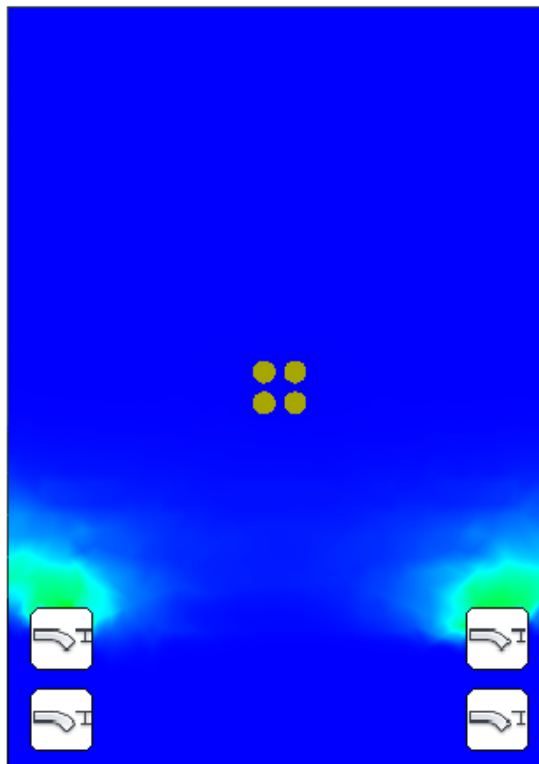
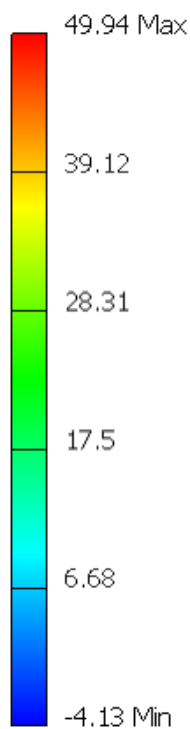
Max. Bending Stress = 49.94N/mm<sup>2</sup> x1.5 = 74.91N/mm<sup>2</sup> < 83.3N/mm<sup>2</sup>

**OK in Bending**

Type: 1st Principal Stress

Unit: MPa

29/04/2020, 12:37:23



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### Glass Analysis - Deflection of Glass Panel due to 1.0kN/m<sup>2</sup> Infill Loading:

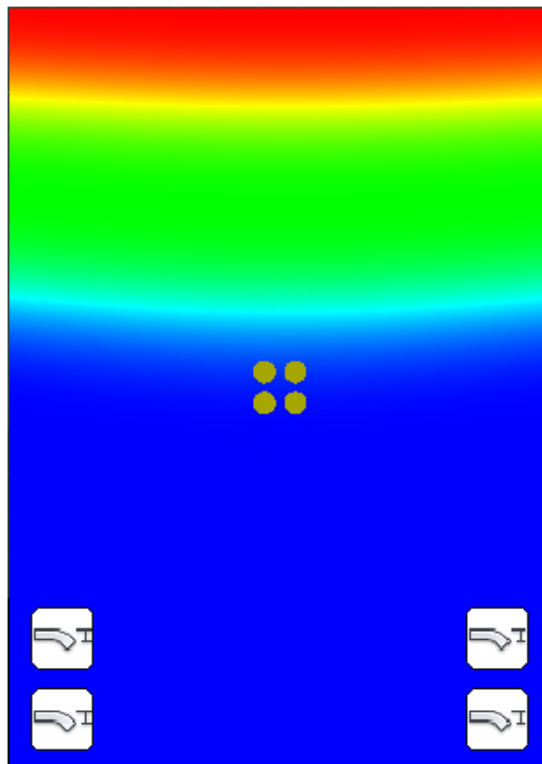
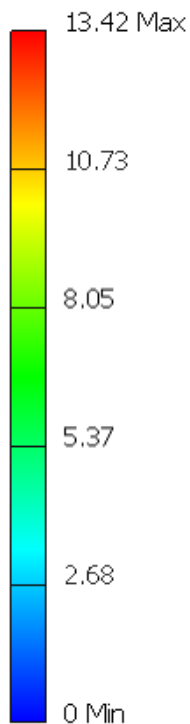
- Analysis Software was used to determine maximum bending stress of the glass due to 1.0kN/m<sup>2</sup> Infill Loading
- 15mm Tough Glass analysed, horizontally toughened Laminated
- Deflection analysed based on glass panel of 1.0m x 1.40m

#### Result:

Max. Deflection = 13.42mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**

Type: Displacement  
Unit: mm  
29/04/2020, 12:37:37



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Case Study 02: 17.52mm (TLT) – 1.0x1.40m – 1.0kN/m<sup>2</sup>:

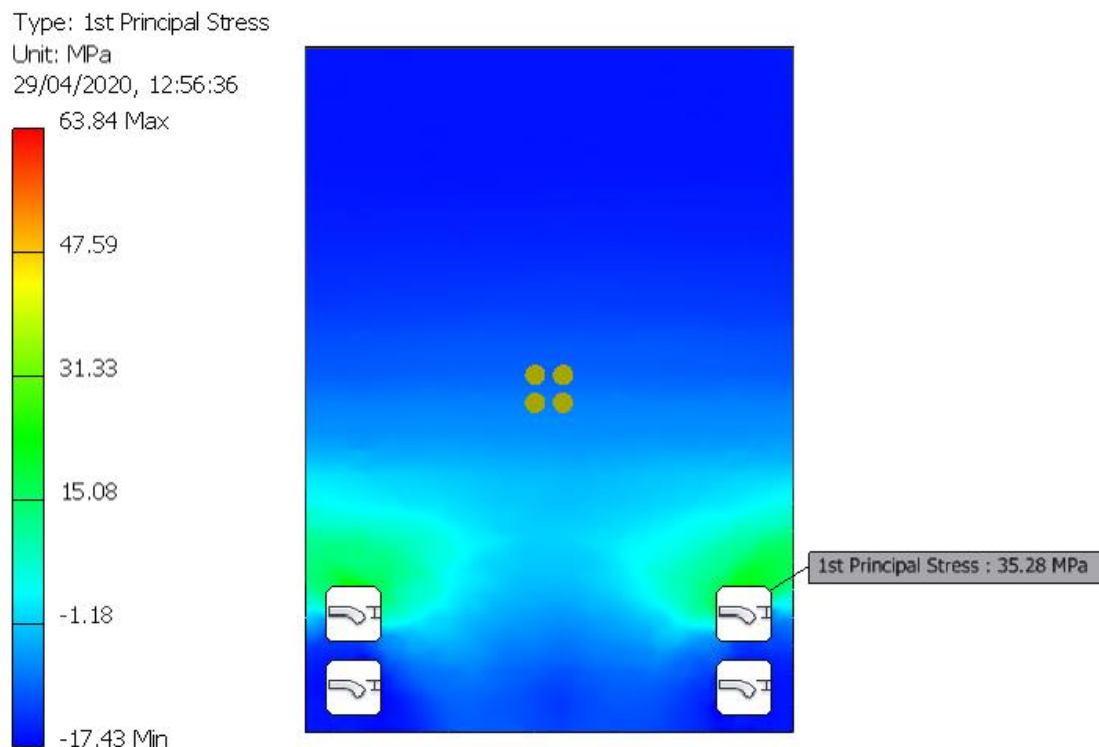
### Glass Analysis - Bending Stress of Glass Panel due to 1.0kN/m<sup>2</sup> Infill Loading:

- Analysis Software was used to determine maximum bending stress of the glass due to 1.0kN/m<sup>2</sup> Infill Loading
- 8/8/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 3MPa, G = 1MPa
- Bending Stress analysed based on glass panel of 1.0m x 1.40m

### Result:

Max. Bending Stress = 35.28N/mm<sup>2</sup> x1.5 = 52.92N/mm<sup>2</sup> < 83.3N/mm<sup>2</sup>

**OK in Bending**



### Note:

In this case the 63.84 MPa is a localised stress. The most appropriate stress to be considered is 35.28 MPa

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### Glass Analysis - Deflection of Glass Panel due to 1.0kN/m<sup>2</sup> Infill Loading:

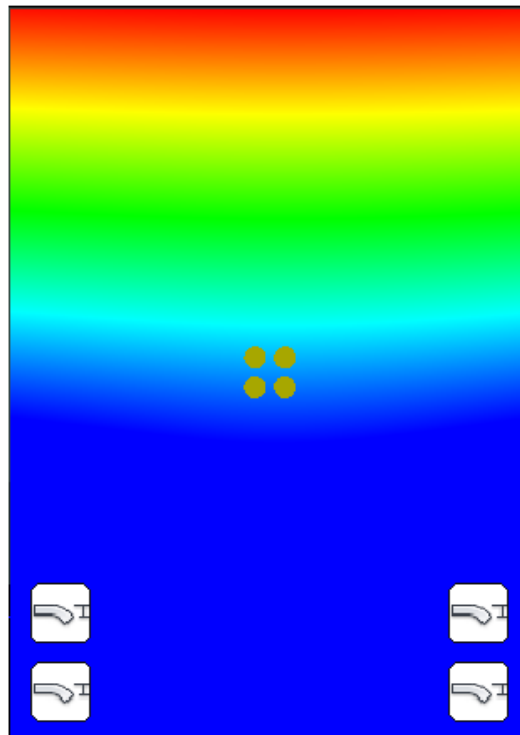
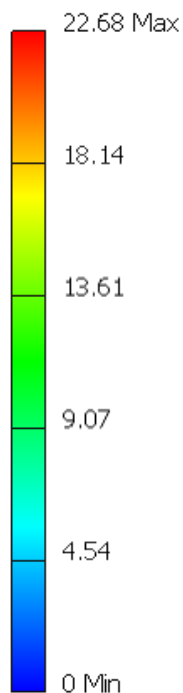
- Analysis Software was used to determine maximum bending stress of the glass due to 1.0kN/m<sup>2</sup> Infill Loading
- 8/8/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 3MPa, G = 1MPa
- Deflection analysed based on glass panel of 1.0m x 1.40m

#### **Result:**

Max. Deflection = 22.68mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**

Type: Displacement  
Unit: mm  
29/04/2020, 12:31:07



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Case Study 03: 21.52mm (TLT) – 1.0x1.40m – 1.5kN/m<sup>2</sup>:

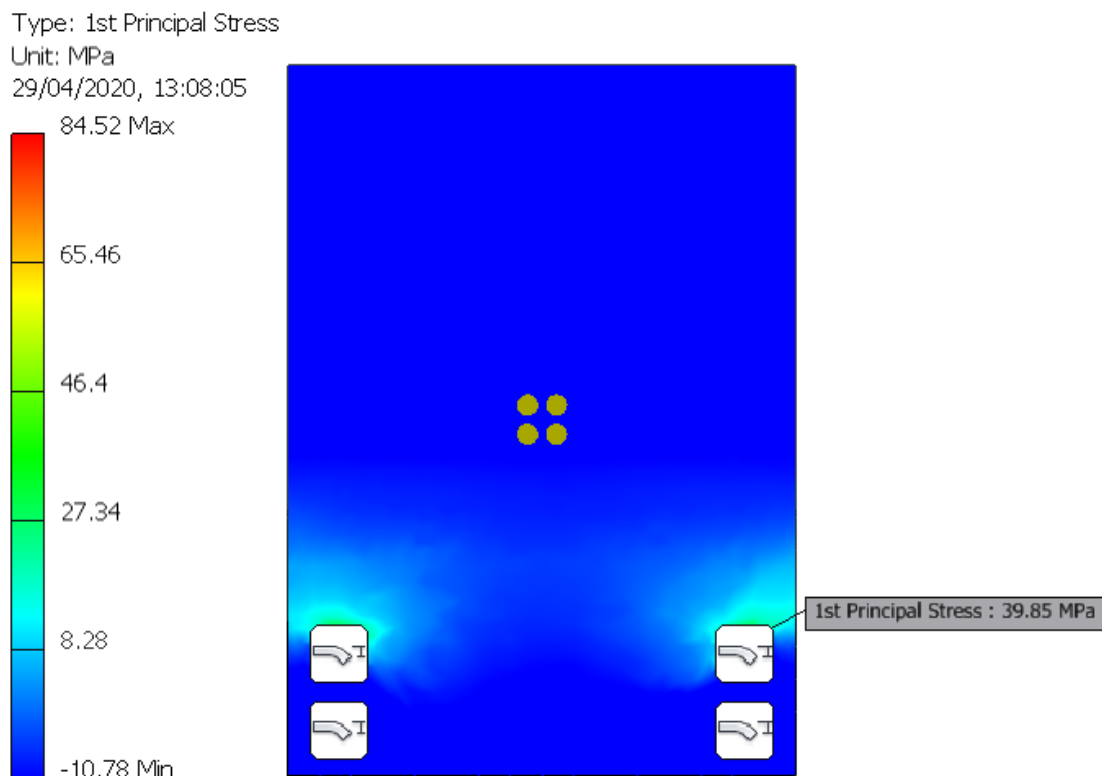
**Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:**

- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m<sup>2</sup> Infill Loading
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 3MPa, G = 1MPa
- Bending Stress analysed based on glass panel of 1.0m x 1.40m

**Result:**

Max. Bending Stress = 39.85N/mm<sup>2</sup> x1.5 = 59.78N/mm<sup>2</sup> < 83.3N/mm<sup>2</sup>

**OK in Bending**



**Note:**

In this case the 84.52 MPa is a localised stress. The most appropriate stress to be considered is 39.85 MPa



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### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:

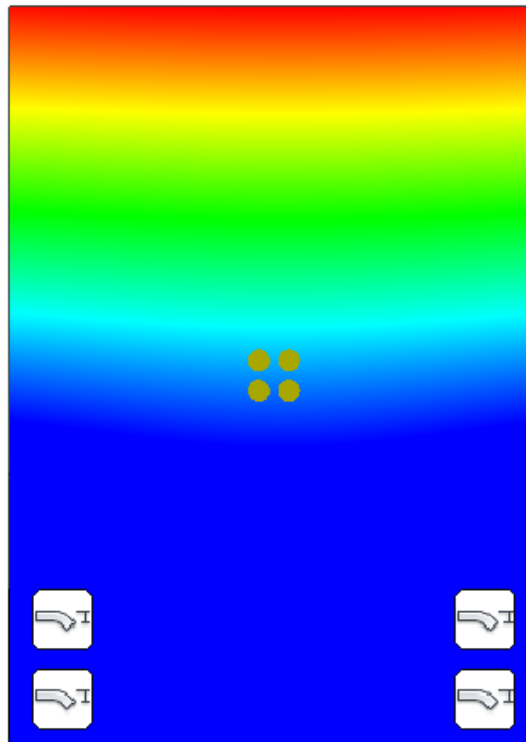
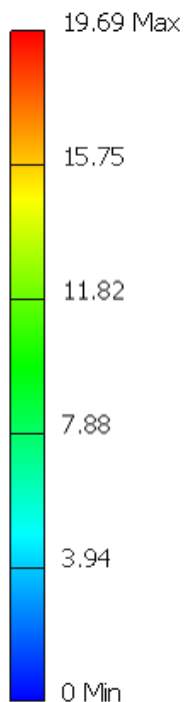
- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m<sup>2</sup> Infill Loading
- 10/10/1.52mm T/L/T Glass analysed, horizontally toughened Laminated
- Interlayer Properties used for analysis, E= 3MPa, G = 1MPa
- Deflection analysed based on glass panel of 1.0m x 1.40m

#### **Result:**

Max. Deflection = 19.69mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**

Type: Displacement  
Unit: mm  
29/04/2020, 13:08:35



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Case Study 04: 15mm Tough – 1.166x1.192m – 0.42kN:

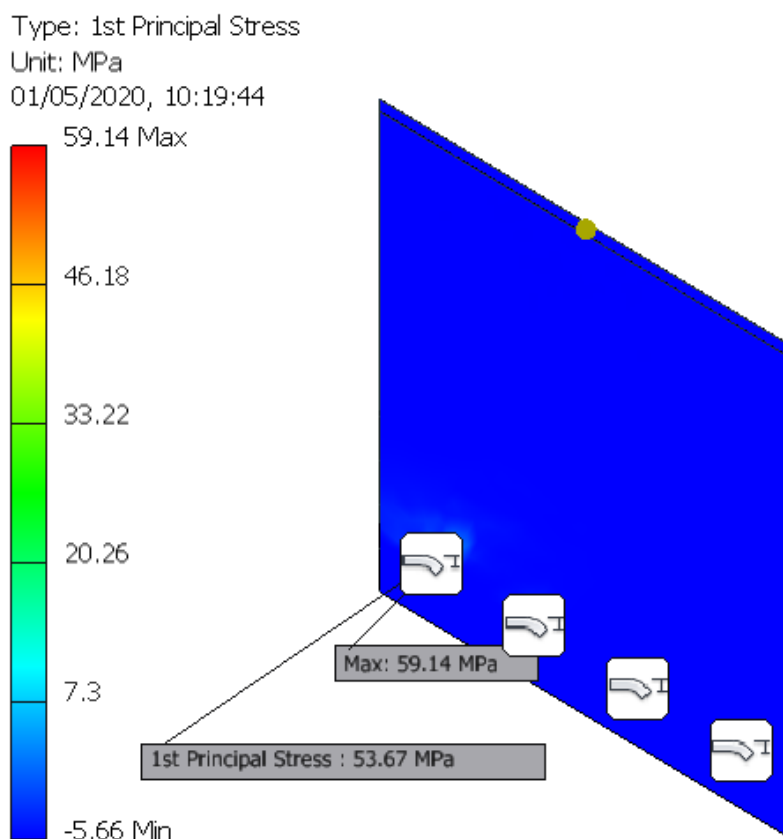
**Glass Analysis - Bending Stress of Glass Panel due to 0.42kN/m Balustrade Loading:**

- Analysis Software was used to determine maximum bending stress of the glass due to 0.42kN/m Balustrade Loading
- 15mm Tough Glass analysed, horizontally toughened Laminated
- Bending Stress analysed based on glass panel of 1.166(l)m x 1.192(h)m

**Result:**

Max. Bending Stress =  $53.67\text{N/mm}^2 \times 1.5 = 80.51\text{N/mm}^2 < 83.3\text{N/mm}^2$

**OK in Bending**



**Note:**

In this case the 59.14 MPa is a localised stress. The most appropriate stress to be considered is 53.67 MPa

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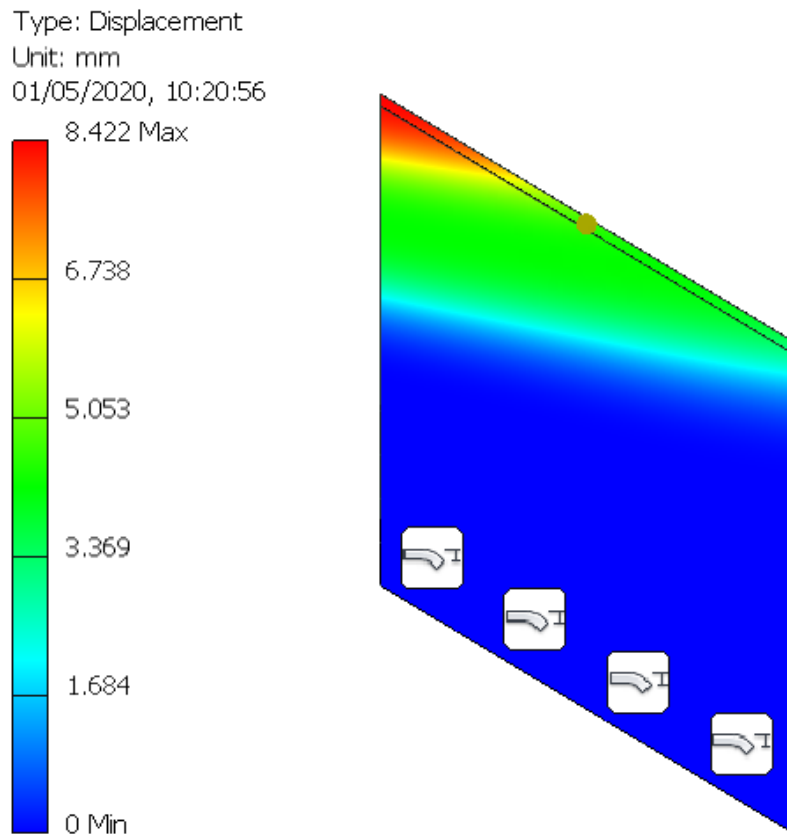
**Glass Analysis - Deflection of Glass Panel due to 0.42kN/m Balustrade Loading:**

- Analysis Software was used to determine maximum bending stress of the glass due to 0.42kN/m Balustrade Loading
- 15mm Tough Glass analysed, horizontally toughened Laminated
- Deflection analysed based on glass panel of 1.166(l)m x 1.192(h)m

**Result:**

Max. Deflection = 8.422mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**



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Case Study 05: 15mm Tough – 1.20x1.11m – 1.5kN/m<sup>2</sup>:

**Glass Analysis - Bending Stress of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:**

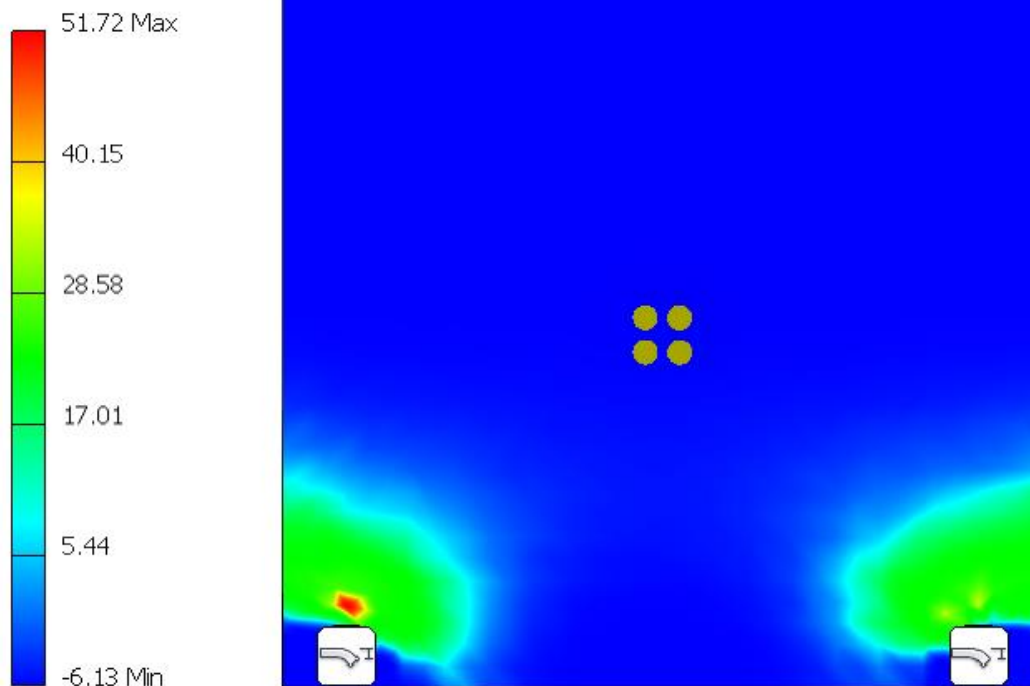
- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m<sup>2</sup> Infill Loading
- 15mm Tough Glass analysed, horizontally toughened Laminated
- Bending Stress analysed based on glass panel of 1.2m x 1.11m

**Result:**

Max. Bending Stress = 51.72N/mm<sup>2</sup> x1.5 = 77.58N/mm<sup>2</sup> < 83.3N/mm<sup>2</sup>

**OK in Bending**

Type: 1st Principal Stress  
Unit: MPa  
01/05/2020, 15:41:55



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### Glass Analysis - Deflection of Glass Panel due to 1.5kN/m<sup>2</sup> Infill Loading:

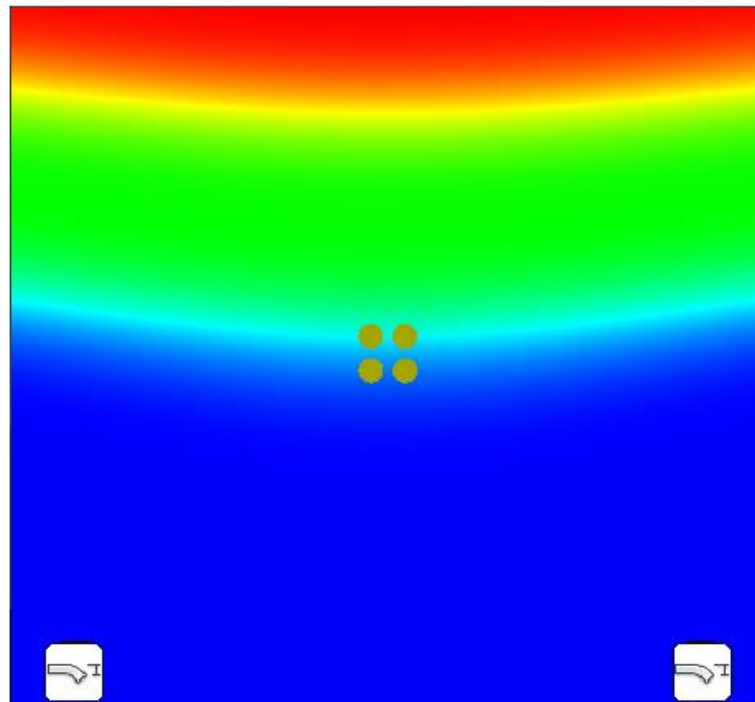
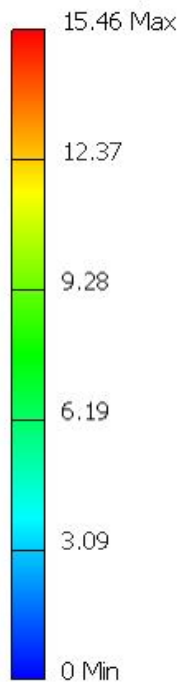
- Analysis Software was used to determine maximum bending stress of the glass due to 1.5kN/m<sup>2</sup> Infill Loading
- 15mm Tough Glass analysed, horizontally toughened Laminated
- Deflection analysed based on glass panel of 1.2m x 1.11m

### Result:

Max. Deflection = 15.46mm < 25mm {BS6180:2011 cl. 6.4.1}

**OK in Deflection (Glass Only)**

Type: Displacement  
Unit: mm  
01/05/2020, 15:40:04





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Reactions:

Size of the Glass - 1.00(h) x 1.40(l)m			
Pressure	Reactions (N)		
	Case 01	Case 02	Case 03
1	1001	1433	2319
2	1001	1433	2319
3	-301	-733	-1269
4	-301	-733	-1269

Size of the Glass	
1.166(h) x 1.192(l)m	
Balustrade	Reactions (N)
	Case 04
1	393
2	26
3	69
4	-68

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### Connection Design:

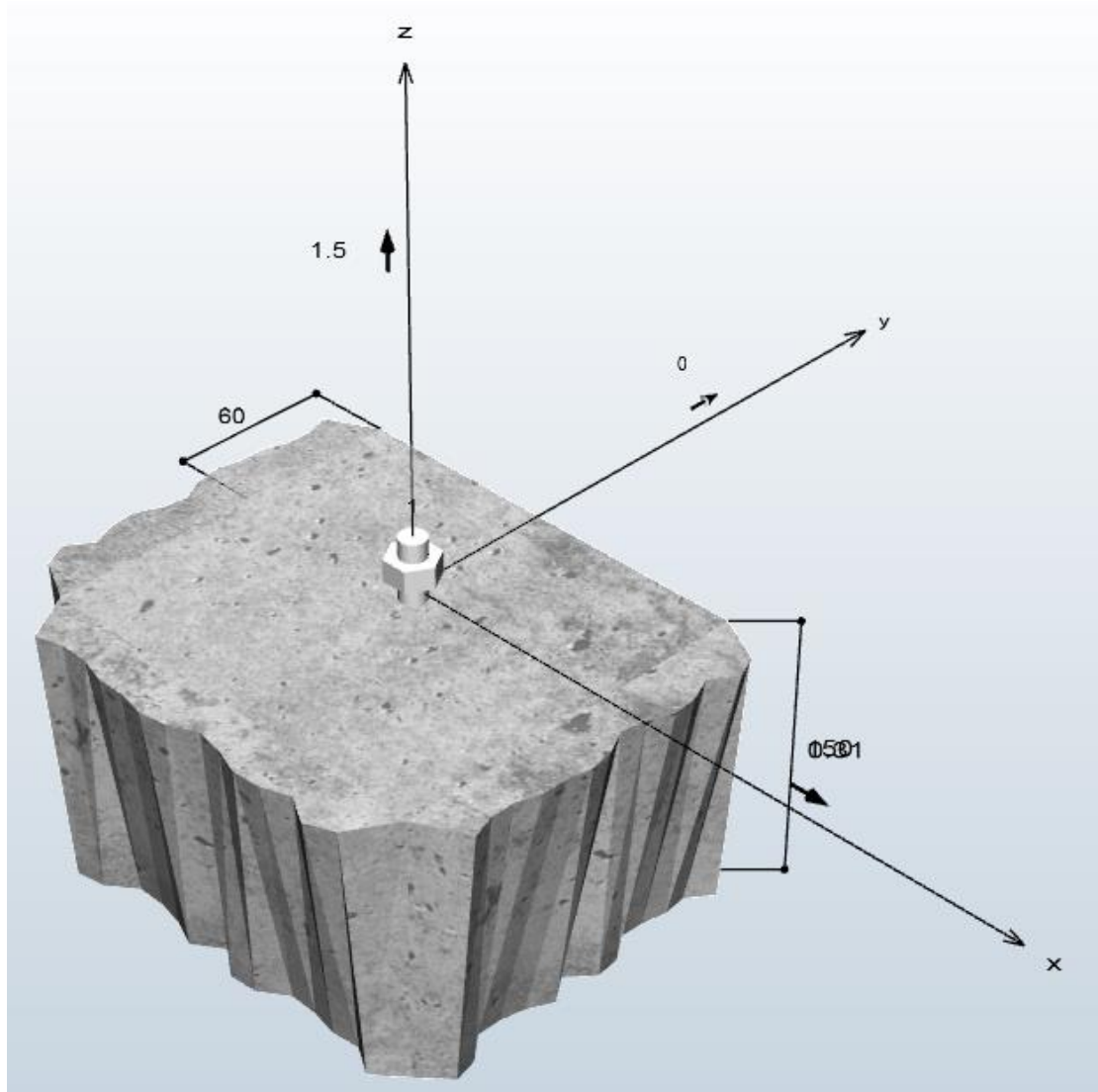
Case Study 01: 15mm Tough – 1.0x1.40m – 1.0kN/m<sup>2</sup>:

#### Connection To Concrete:

Tensile Load = 1.00kN × 1.5 = 1.50kN (ULS)

Shear Load = 0.23kN × 1.35 = 0.31kN (ULS)

**Chemical anchor FIS V 360 S M10x150 8.8. See design in Appendix A.**



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Connection To Mild Steel:

1Nr M10 Bolt Grade 8.8

$$f_y = 210 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 500 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$A = 58 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

$$F_{t,Ed} = \frac{1.0 \text{ kN}}{\text{m}^2} \times 1.5 \times 1.4 \text{ m} \times 1.0 \text{ m} \times \frac{1.4 \text{ m}}{2} = 1.47 \text{ kNm}$$

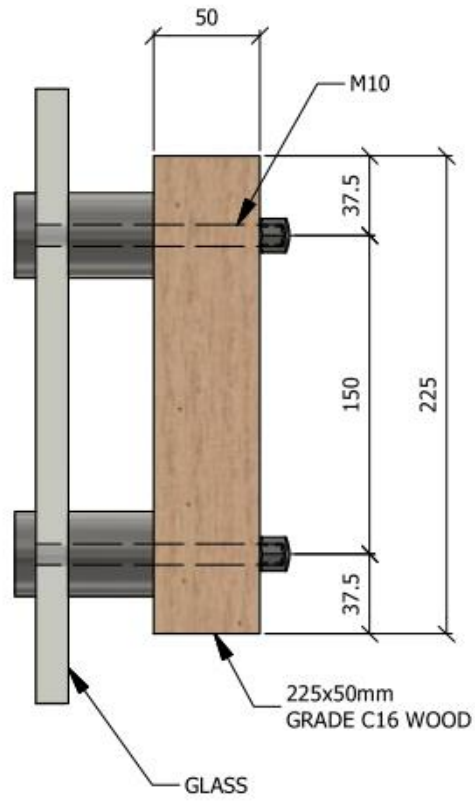
$$F_{t,Ed} = \frac{1.47 \text{ kN}}{0.15 \text{ m}} \times \frac{1}{2} = 4.9 \text{ kN per bolt}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} \rightarrow F_{t,Rd} = \frac{0.9 \times 500 \times 58 \times 10^{-3}}{1.25} = 20.88 \text{ kN} > 4.9 \text{ kN} \quad \text{Okay}$$



<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 25
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Connection To Wood:



<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.:</b> 26
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

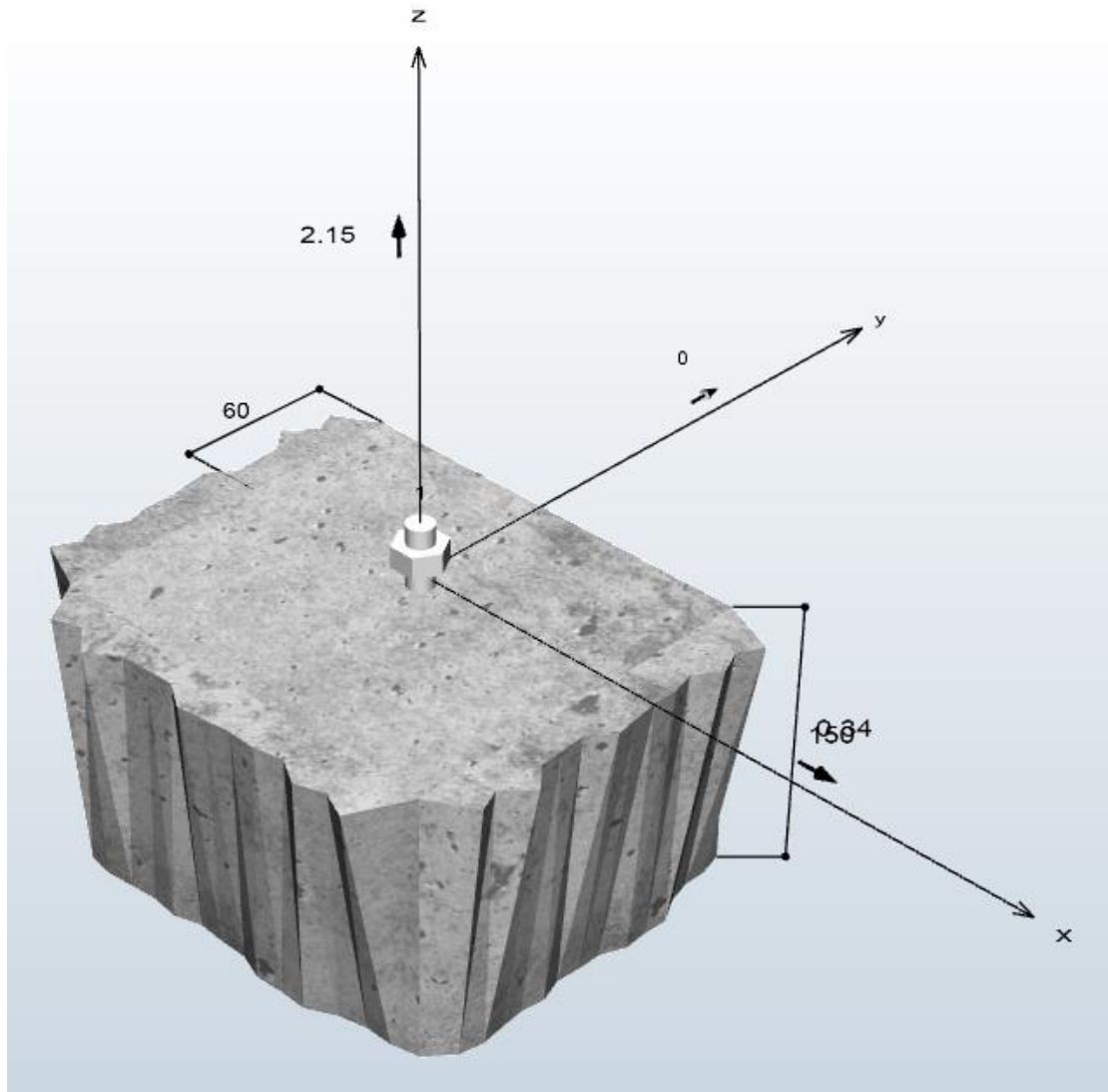
Case Study 02: 17.52mm (TLT) – 1.0x1.40m – 1.0kN/m<sup>2</sup>:

Connection To Concrete:

Tensile Load = 1.43kN × 1.5 = 2.15kN (ULS)

Shear Load = 0.26kN × 1.35 = 0.34kN (ULS)

**Chemical anchor FIS V 360 S M10x150 8.8. See design in Appendix A.**



<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.:</b> 27
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Mild Steel:

1Nr M10 Bolt Grade 8.8

$$f_y = 210 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 500 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$A = 58 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

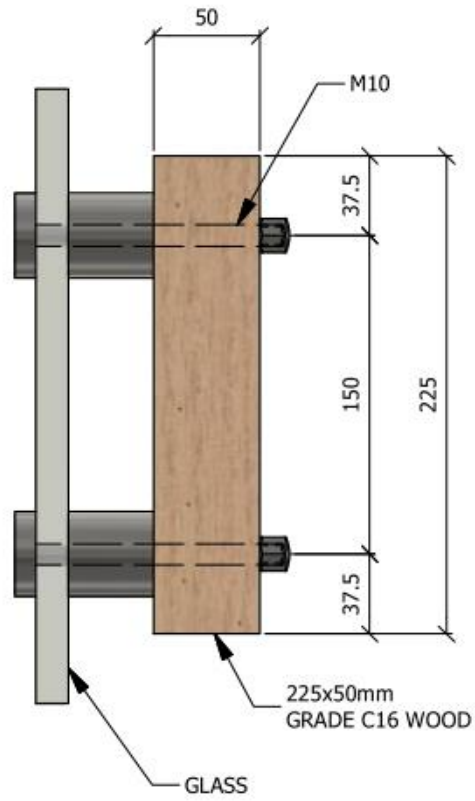
$$F_{t,Ed} = \frac{1.5 \text{ kN}}{\text{m}^2} \times 1.5 \times 1.4 \text{ m} \times 1.0 \text{ m} \times \frac{1.4 \text{ m}}{2} = 2.21 \text{ kNm}$$

$$F_{t,Ed} = \frac{2.21 \text{ kN}}{0.15 \text{ m}} \times \frac{1}{2} = 7.37 \text{ kN per bolt}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} \rightarrow F_{t,Rd} = \frac{0.9 \times 500 \times 58 \times 10^{-3}}{1.25} = 20.88 \text{ kN} > 7.37 \text{ kN} \quad \text{Okay}$$

<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 28
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Wood:



**FIS A M 10x150 8.8.**

<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.:</b> 29
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

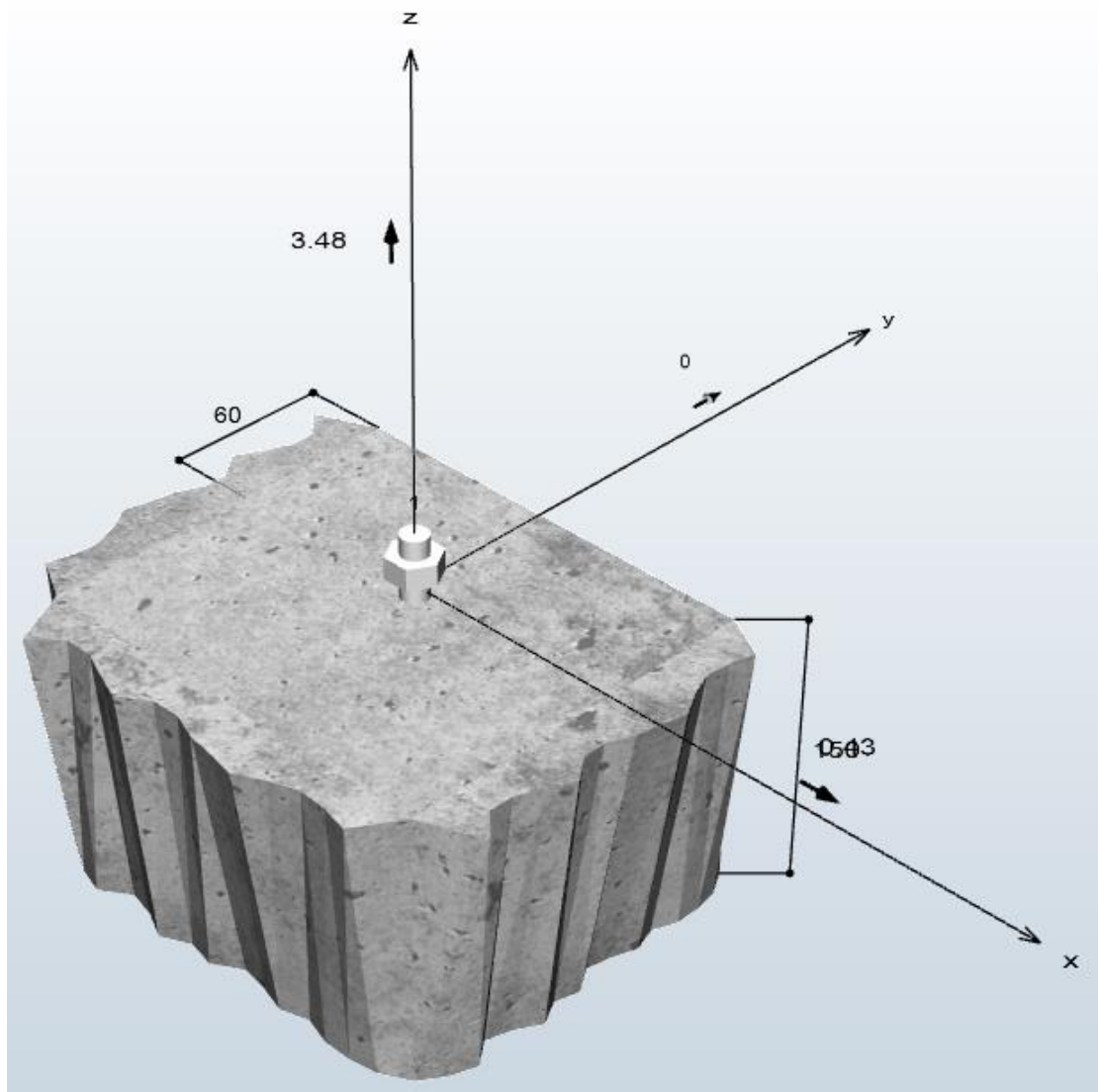
Case Study 03: 21.52mm (TLT) – 1.0x1.40m – 1.5kN/m<sup>2</sup>:

Connection To Concrete:

Tensile Load =  $2.32\text{kN} \times 1.5 = 3.48\text{kN}$  (ULS)

Shear Load =  $0.32\text{kN} \times 1.35 = 0.43\text{kN}$  (ULS)

**Chemical anchor FIS V 360 S M10x150 8.8. See design in Appendix A.**



<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.:</b> 30
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Mild Steel:

1Nr M10 Bolt Grade 8.8

$$f_y = 210 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 500 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$A = 58 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

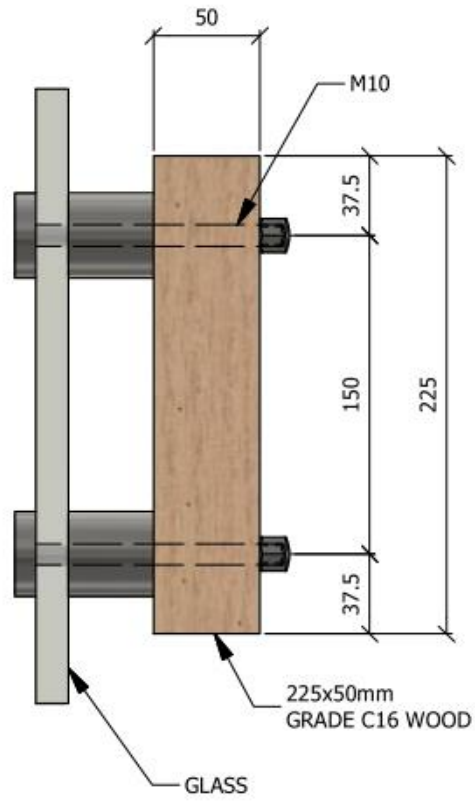
$$F_{t,Ed} = \frac{2.0 \text{ kN}}{\text{m}^2} \times 1.5 \times 1.4 \text{ m} \times 1.0 \text{ m} \times \frac{1.4 \text{ m}}{2} = 2.94 \text{ kNm}$$

$$F_{t,Ed} = \frac{2.21 \text{ kN}}{0.15 \text{ m}} \times \frac{1}{2} = 9.80 \text{ kN per bolt}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} \rightarrow F_{t,Rd} = \frac{0.9 \times 500 \times 58 \times 10^{-3}}{1.25} = 20.88 \text{ kN} > 9.80 \text{ kN} \quad \text{Okay}$$

<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 31
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Connection To Wood:



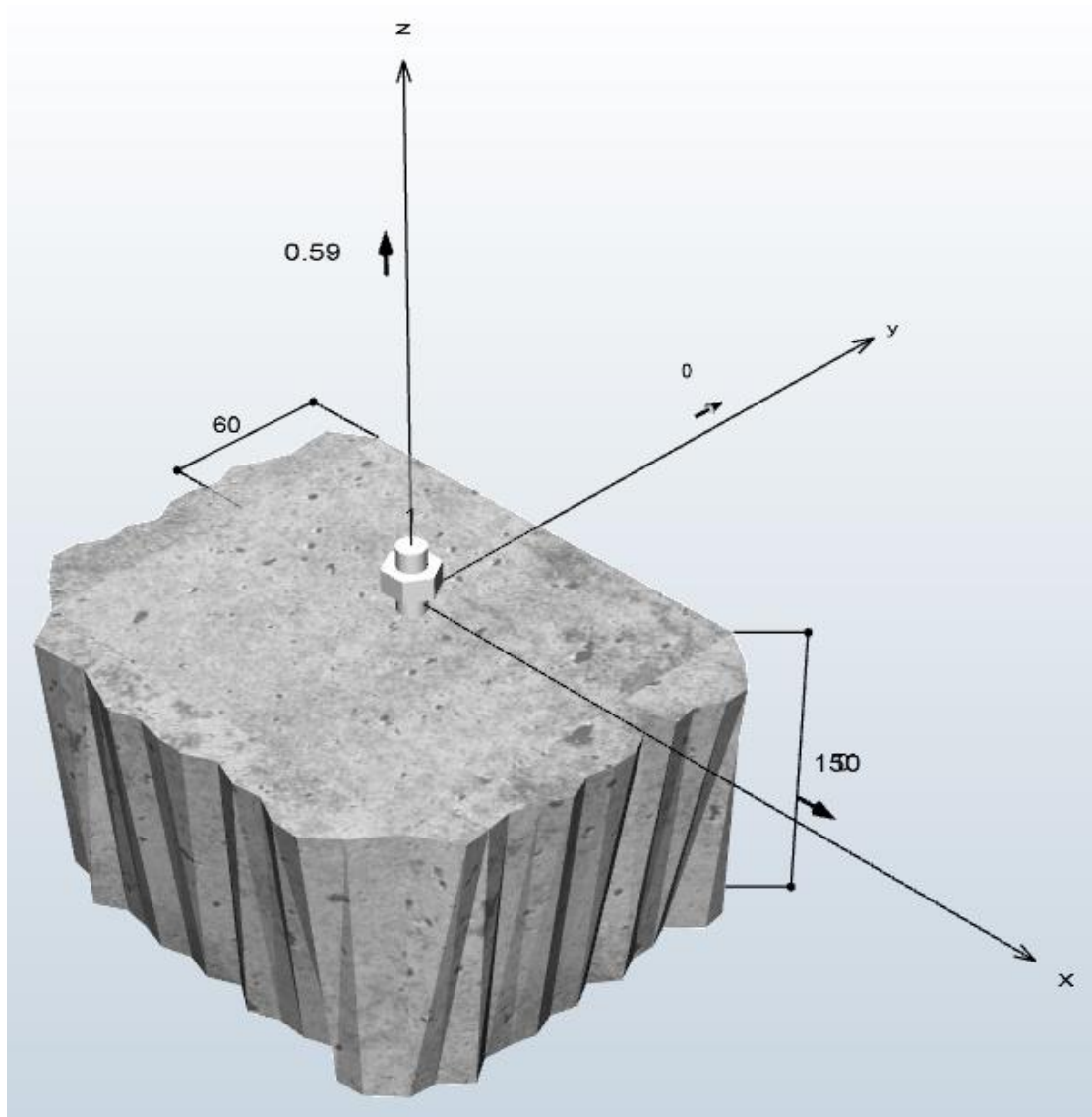
<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.:</b> 32
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Case Study 04: 15mm Tough – 1.166x1.192m – 0.42kN:

Connection To Concrete:

Tensile Load =  $0.393\text{kN} \times 1.5 = 0.59\text{kN}$  (ULS)

**Chemical anchor FIS V 360 S M10x150 8.8. See design in Appendix A.**





<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 33
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Mild Steel:

1Nr M10 Bolt Grade 8.8

$$f_y = 210 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 500 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$A = 58 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

$$F_{t,Ed} = 0.393 \text{ kN} \times 1.5 = 0.59 \text{ kN (ULS)}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} \rightarrow F_{t,Rd} = \frac{0.9 \times 500 \times 58 \times 10^{-3}}{1.25} = 20.88 \text{ kN} > 0.59 \text{ kN} \quad \text{Okay}$$

<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 34
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Wood:

$$f_y = 210 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.1 EN 1993-1-4:2006})$$

$$f_{ub} = 500 \text{ MPa} \quad (\text{Grade 304 Stainless Steel, Table 2.2 EN 1993-1-4:2006})$$

$$\alpha = 0.6 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$A = 58 \text{ mm}^2 \quad (\text{For M10 Bolts})$$

$$K_2 = 0.9 \quad (\text{Table 3.4 EN 1993-1-8:2005})$$

$$\lambda_{m2} = 1.25 \quad (\text{Table 5.1 EN 1993-1-4:2006})$$

Tensile Resistance Check: (Table 3.4 EN 1993-1-8:2005)

$F_{t,Ed}$ : is the design tensile force per bolt for the ultimate limit state.

$F_{t,Rd}$ : is the design tension resistance per bolt.

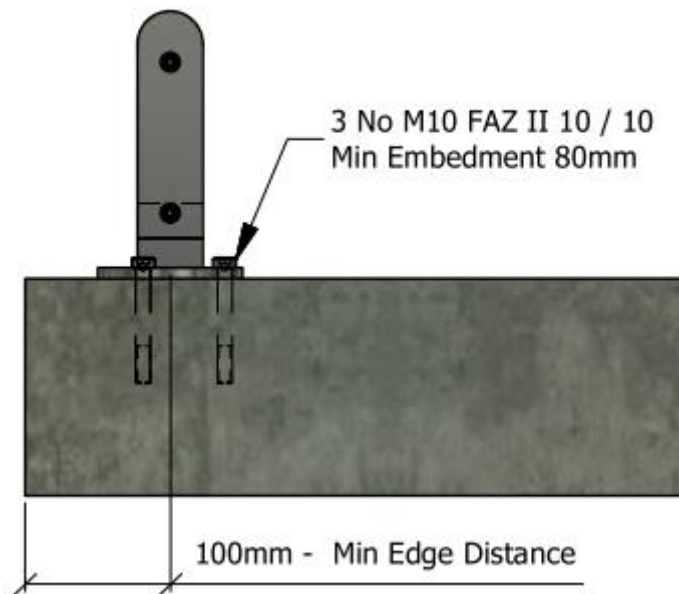
$$F_{t,Ed} = 0.393 \text{ kN} \times 1.5 = 0.59 \text{ kN (ULS)}$$

$$F_{t,Rd} = \frac{K_2 F_{ub} A}{\lambda_{m2}} \rightarrow F_{t,Rd} = \frac{0.9 \times 500 \times 58 \times 10^{-3}}{1.25} = 20.88 \text{ kN} > 0.59 \text{ kN} \quad \text{Okay}$$

<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 35
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

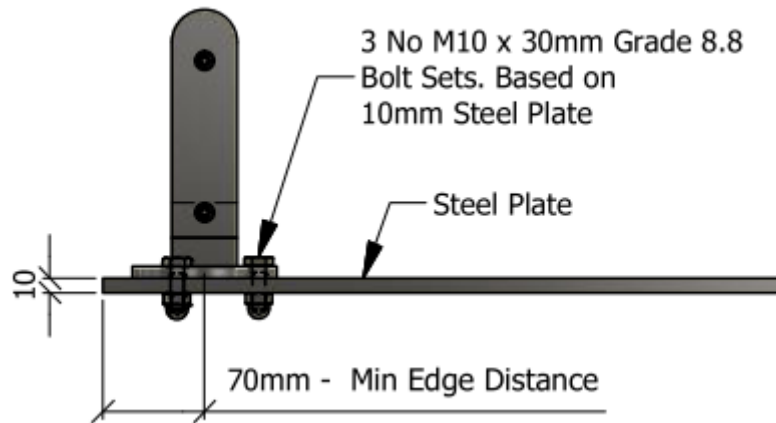
Case Study 05: 15mm Tough – 1.20x1.11m – 1.5kN/m<sup>2</sup>:

Connection To Concrete:



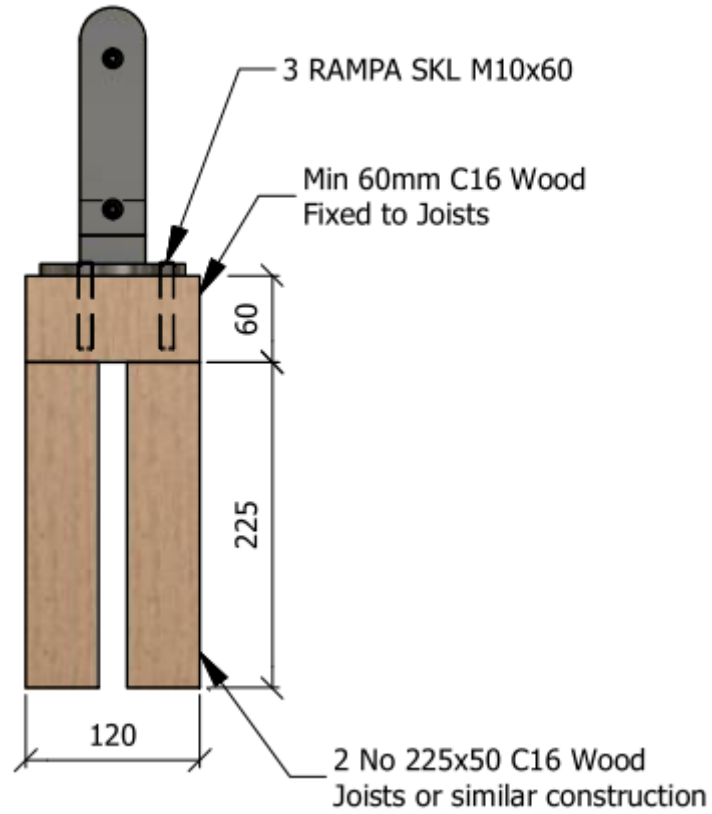
<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 36
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Mild Steel:



<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 37
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

Connection To Wood:





<b>Project:</b> Glass Adaptor and Spigot	<b>Contract:</b> 1388-3
<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 38
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

## Appendix A - Fiscer Reports

TSA is Both the Designer and the Specifier of the Fixings.



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<b>Subject:</b> General Wind Load	<b>Sheet No.</b> 39
<b>Date:</b> 08/05/2020	<b>By:</b> R.F.

## Appendix B – Rampa

TSA is Specifier of the Fixings